



Memo

To: Daniel Ford (MPDC)
From: Kevin Hoang (WSP, Geotechnical Engineer)
Technical Review: Tristan McWilliam (WSP, Principal Geotechnical Engineer – Geotech Lead)
Subject: **Macquarie Point Multipurpose Stadium, Bulk Earthworks – Impact Assessment on Road Reserve**
Our ref: PS225332-HOB-GEO-MEM-010 RevC
Date: **21 April 2026**

Dear Daniel,

This memorandum has been prepared in response to the request from MPDC to address the requirements of The Order, for the approval of the early works associated with the Macquarie Point Multipurpose Stadium Development. The Order requires the assessment of the impact of bulk earthworks on the structural integrity of the road reservation during construction (item B13). To address this requirement, WSP has assessed the global stability of selected sections through the proposed excavation batters assessed to be critical. If the assessed factors of safety meet the target minimum values typically adopted in engineering practice, then it is likely that there will be no significant ground movement and no significant impact to the road reserve infrastructure. Any ground movement resulting from the removal of material during bulk excavation is assessed to be not significant.

WSP has been responsible for the geotechnical investigation and geotechnical interpretation for the proposed development, including both review of historical investigations undertaken on the site and undertaking additional investigations to inform data gaps.

Summary of the assessment

The geotechnical analyses undertaken to assess the requirements of The Order (Item B13) indicate the proposed bulk earthworks will not adversely affect the structural integrity of the road reservation. Refer to the sections below for the information considered, process adopted and assumptions made in the assessment.

An addendum memorandum containing the outputs of all analyses undertaken is provided separately (reference PS225332-HOB-GEO-MEM-011 revA).

Available information

The assessment has considered the following information:

- Rarein, 2026, *MPDC - Bulk Earthworks Plan & Sections RevA*, Project No. 246011, Dwg: C121, 141-142, Issued 16 February 2026 (Attachment A)
- WSP, 2026, *Macquarie Point Stadium Development Additional Site Investigation – Geotechnical Interpretive Report (GIR) Rev0*, PS225332-WSP-HOB-GEO-REP-006 Rev0, Issued 30 January 2026.

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WSP acknowledges that every project we work on takes place on First Peoples lands.
We recognise Aboriginal and Torres Strait Islander Peoples as the first scientists and engineers and pay our respects to Elders past and present.

Compliance Requirements

The compliance requirements of B13 as supplied to WSP are as follows:

Before a stage of works for the Project commences, detailed design drawings, structural certificates and associated geotechnical assessments for that stage must be prepared that:

- (a) are prepared by a suitability qualified and experienced engineer and include:
 - (i) confirming whether the works have, or do not have, an impact on the structural integrity of the road reservation during construction or operation; and
 - (ii) whether the works comply with AS 4678-2002 and specify a design life, consistent with Table 3.1 of that Standard, for major public infrastructure relating to the works; and
 - (iii) information that accounts for any additional surcharge loadings in accordance with relevant standards prepared by Standards Australia; and
 - (iv) a reference, and response, to any relevant geotechnical investigations or findings; and
 - (v) any necessary mitigation measures to protect road infrastructure; and
 - (vi) details on the location and structural design of footings adjacent to the highway reservation; and
 - (vii) a requirement that all works must be performed in accordance with this order; and
- (b) are approved by the Secretary State Growth before works on that stage commence.

Critical Sections

Based on the bulk excavation drawings and sections provided (see Appendix A), the proposed batters for bulk excavation are 2H:1V or flatter. Figure 1 provides a sketch of the proposed excavation geometry relative to the road infrastructure. The greater the offset distance from the footpath to the bulk excavation, the lower the risk of the road infrastructure being affected. The flatter the temporary batter, shown below as 2(Horizontal):1(Vertical), the lower the risk of instability affecting the road infrastructure.

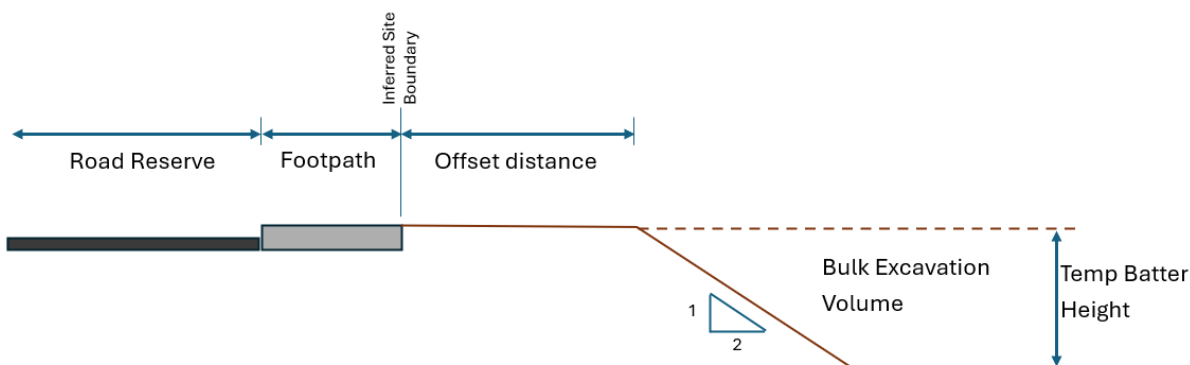


Figure 1 Example Sketch of Excavation relative to the Road Reserve and Footpath

We have identified that:

- At the southern side of the site footprint, the extents of the bulk excavation are adjacent to Evans Street. The proposed bulk excavation drawings indicate that the crest of temporary batters is offset approximately 4.7 m from the footpath. The global stability of this section has been assessed.
- At the northern side of the site footprint, the proposed bulk excavation is approximately 12 m from the existing road (Peace Promenade). We note that the Peace Promenade is likely to be demolished as part of the works, however the impact on it has been assessed in the event the road remains in service during bulk excavation works.
- At the western side of the site footprint, the bulk excavation is approximately 20 m away from Tasman Highway and it is highly unlikely to impact the road reservation. No analysis is required for the western batter.

- At the eastern side of the works, the bulk excavation footprint is approximately 60 m away from Hunter Street and it is highly unlikely to impact other structures or the road reservation. No analysis is required for the eastern batter.

Ground profiles

The ground model adopted for the analysis of the Southern Area is summarised in Table 1.

Table 1 Expected ground conditions – Southern Area (near Evans St)

Material	Depth Encountered	Top Surface Elevation	Thickness Range
Fill (granular or cohesive)	0 m	3 m to 4 m AHD	5 m to 9 m
Estuarine Soils (granular)	5 m to 9 m	-1 to -5 m AHD	5 m to 10 m

Materials deeper than the estuarine unit are outside the zone of influence of earthworks operations.

Due to the variability of the subsurface material on the site, we have considered a number of different ground conditions, to model the range of conditions that may be encountered. Several ground model configurations have been tested as a sensitivity check for potentially varying ground conditions across the site. The ground models assessed are as follows:

- A fill profile comprising granular or cohesive material to a depth of 10 m or greater,
- A fill profile comprising granular or cohesive material to a depth of 5 m, overlying estuarine soils.

The ground model adopted for the analysis of the Northern Area is summarised in Table 2. A review of the ground conditions to the north indicate that the bulk excavations and temporary batters will likely be cut into weathered Dolerite along most of the section. To the northeast, one of the proposed temporary batters that is 4 m high will be excavated in fill that is underlain by Dolerite bedrock. This section has been considered in the analysis.

Table 2 Expected ground conditions – Northern Area (near Peace Promenade)

Material	Depth Encountered	Elevation	Thickness Range
Fill	0 m	6 m AHD	0 m to 6 m
Dolerite (moderately to highly weathered)	0 m to 6 m	0 m to 6 m AHD	0 m to 7 m
Dolerite (slightly weathered to fresh)	0 m to 7.5 m	-2.5 m to 0 m AHD	Not proven

Two ground model configurations have been tested as a sensitivity check for potentially varying ground conditions in the northern area of site. The most prevalent ground models consist of:

- A fill profile comprising granular material overlying weathered Dolerite, and
- A fill profile comprising cohesive material overlying weathered Dolerite.

Groundwater

A design groundwater level of RL 2 m AHD for the Southern Area of the site has been adopted for the analysis, following review of information provided in the GIR. A design groundwater level of RL 3 m AHD upslope has been adopted for short term stability cases, accounting for ground conditions following prolonged heavy rainfall resulting in an elevated groundwater table.

In the Northern Area of site, a design groundwater level of RL 2.4 m AHD for the northern area of the site has been adopted for the analysis. This level considers some dewatering that may be required to facilitate excavation. A design groundwater level of RL 4 m AHD upslope, in line with groundwater level readings from a nearby monitoring well, has been adopted for short term stability cases, accounting for ground conditions following prolonged heavy rainfall, resulting in an elevated groundwater table.

Geotechnical design parameters

Geotechnical design parameters are consistent with the values adopted in the Geotechnical Interpretive Report (reference PS225332-WSP-HOB-REP-006 rev0 dated 30 January 2026). The values are provided in Appendix B.

Geometry & Loading

Sensitivity checks were undertaken to consider different offset values of pedestrian and road traffic loading from the crest of the temporary batters. Pedestrian and road traffic loading have been assumed at 10 kPa and 20 kPa respectively. No surcharge has been assessed at the crest of the slopes. The modelled cases included assessment of the following:

Southern Area

- Temporary batter height: 1.6 m (base case) and 2.0 m (sensitivity).
- Offset from pedestrian footpath: 4.7 m (base case), 0.0 m and 2.0 m offset (sensitivity).
- Slope angle: 2H:1V.

Northern Area

- Temporary batter height: 4.2 m (base case).
- Offset from pedestrian footpath: 12.0 m (base case), 2.0 m offset (sensitivity).
- Slope angle: 2H:1V (base case) and 2.5H:1V (sensitivity).

Stability Assessment

The abovementioned cases were assessed for the following scenarios:

- “Longer term” stability: longer term groundwater conditions and effective stress parameters adopted to represent conditions and geometry that may exist for about 6-12 months. The works are still temporary in nature and can be maintained during construction.
- Short term stability: short term and elevated groundwater conditions, expected to represent conditions that may exist for days to weeks in duration. The works are temporary in nature and can be maintained during construction.
- Earthquake loading (EQ): short term conditions adopting horizontal acceleration coefficient of 0.104, representing conditions lasting seconds in duration.

Minimum slip surface depths of 0.5 m have been assumed. The following factor of safety criteria for the proposed temporary batters is summarised in Table 3.

Table 3 *Geotechnical Stability Target Factor of Safety values*

Scenario	Target Minimum Factor of Safety
“Longer term” stability	1.2
Short term stability	1.2
Earthquake loading	1.1

Results of Analysis

Stability analyses were performed using SLOPE/W, with factors of safety calculated in accordance with the Morgenstern–Price limit equilibrium method. The results summary table for the design geometries and ground conditions assessed is provided in Appendix C. Figure 2 and Figure 3 provide example cross sections of the base case analyses. The assessed circular slip plane for each geometry is shown in green. The analysis outputs for all cases assessed are provided in an addendum report reference PS225332-HOB-GEO-MEM-011 revA.

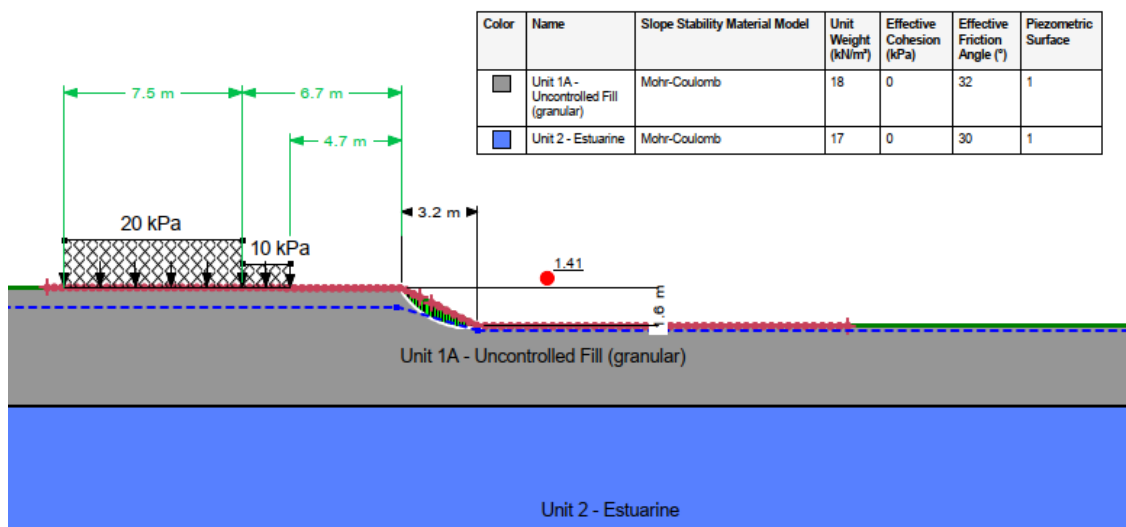


Figure 2 Example Stability Assessment for the Southern Area (Evans St)

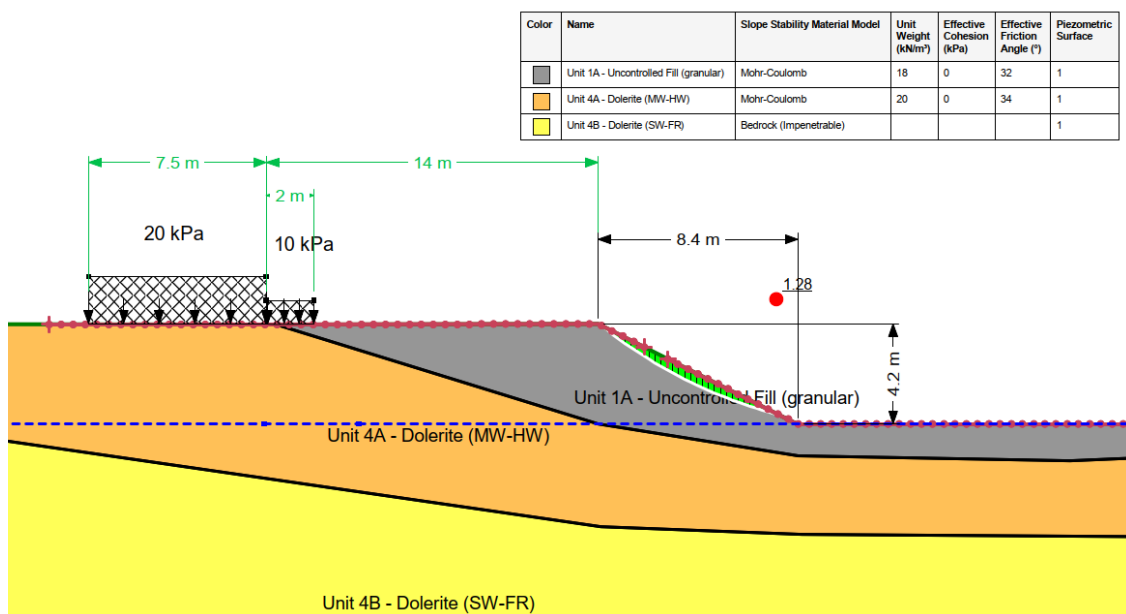


Figure 3 Example Stability Assessment for the Southern Area (Evans St)

The results indicate the following for the sections assessed:

- The Longer Term and Short Term stability cases meet the target minimum factor of safety of 1.2.
- The Earthquake analyses for failure surfaces affecting the road reserve and pedestrian footpath meet the target minimum factor of safety of 1.1.
- Some shallow slip surfaces in granular fill assessed under earthquake loading indicate marginally stable factor of safety values (i.e. between 1.0 and 1.1), however these slip surfaces do not affect the road reserve.

Sensitivity analyses have also been undertaken to assess the impact to the road and footpath if the height of the temporary batters or offsets from the batter crest to the road and footpath reservation are changed. The results summary table for the sensitivity analyses are provided in Appendix C.

Conclusions & Recommendations

The stability analysis indicates that the target minimum factors of safety for the identified geometries are achieved. On this basis, we assess that there is likely to be negligible ground movement of the road reserve and footpaths that could lead to adverse performance of the infrastructure.

With regard to the B13 clause items:

- (a)(i) The bulk excavation works do not impact the structural integrity of the road reserve during construction.
- (a)(ii) AS4678-Earth Retaining Structures is not relevant for the proposed bulk excavation works as no retaining structures are proposed within this phase.
- (a)(iii) The assessment has considered surcharge loading for vehicle and pedestrian loading, additional loading closer to the batter has been considered in the sensitivity cases.
- (a)(iv) The ground models have been adopted from the Geotechnical Interpretive Report prepared for the site.
- (a)(v) Additional mitigation measures to be implemented are discussed below.
- (a)(vi) The consideration of loading from structural elements is not relevant for this stage of the works.
- (a)(vii) The bulk excavation earthworks constructed in accordance with the provided documentation and this technical memorandum are assessed to meet the requirements of the B13 order.

The sensitivity analysis indicates that the excavation can be positioned at 2 m offset from the road reserve (including footpath) and meet the stability requirements of The Order with regard to impact on infrastructure. We recommend that a 2 m offset provides a practical buffer to significantly reduce the likelihood of the excavation plant interacting or damaging the road reserve infrastructure. Should surcharge loading be required to be applied within the offset distance, a site specific assessment should be undertaken.

Where temporary batters are exposed for significant periods of time, they are at risk of erosion from rain and surface water flow. Measures that can be implemented to help protect temporary batters in soil from erosion include:

- Flattening of the batters.
- Temporary batter vegetation to prevent soil erosion.
- Application of erosion control blankets, coir logs or geotextile wraps.
- Drainage controls and diversions of water from the crest and toe of the batter.



Limitations

Refer to the limitations statement in Appendix D for the uses of this technical memorandum.

A handwritten signature in black ink, appearing to read 'Tristan McWilliam'.

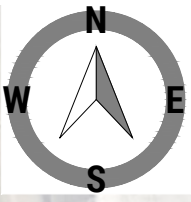
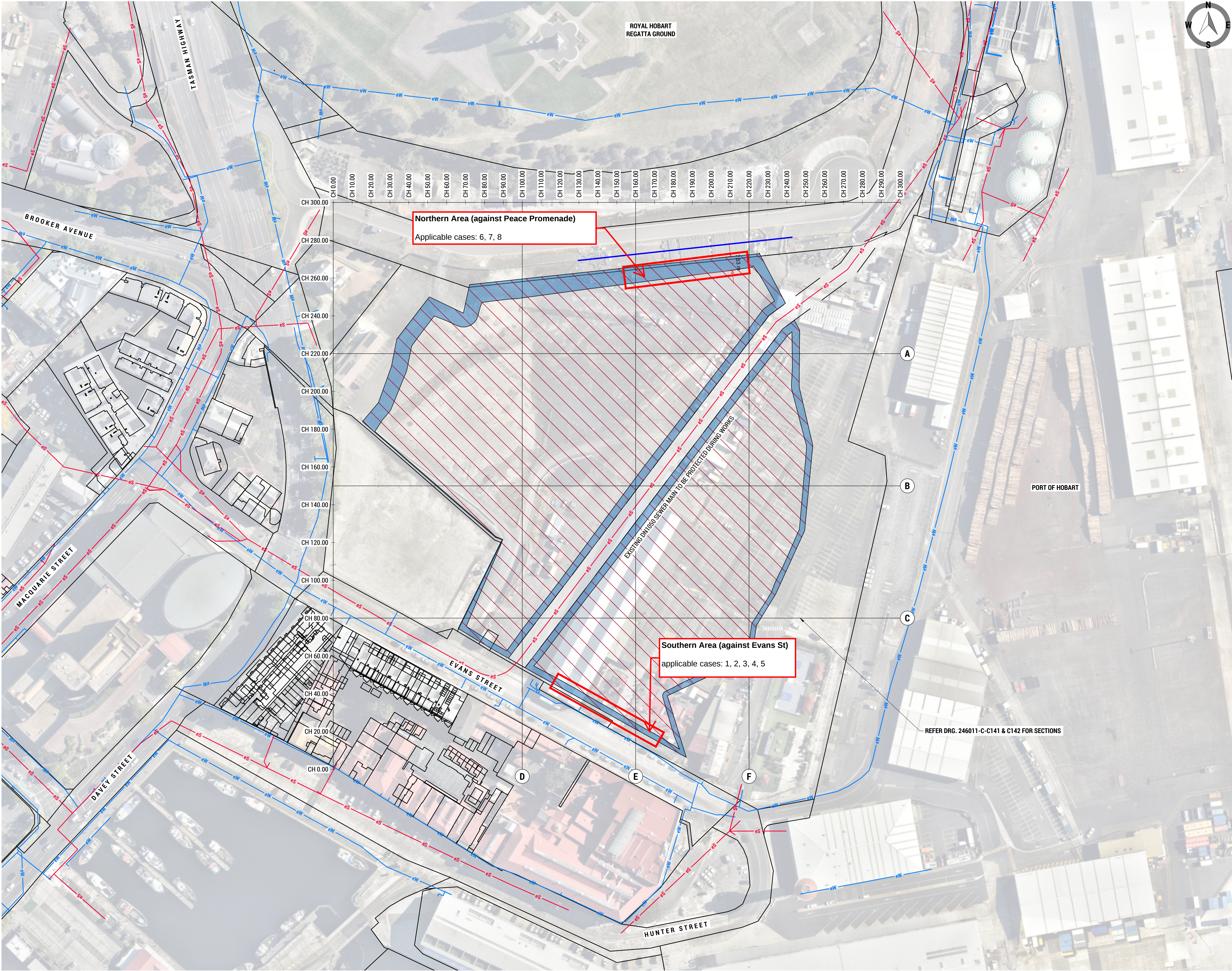
Tristan McWilliam
Principal Geotechnical Engineer
BEng(Hons), MEngSc, MIEAust, CPEng, NER

Appendices

Appendix A	Bulk Excavation Drawings and Sections
Appendix B	Design Parameters
Appendix C	Results Summary
Appendix D	Limitations Statement



Appendix A Bulk Excavation Drawings and Sections

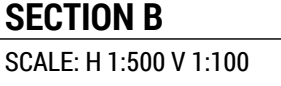


- NOTES**
- WORKS TO BE IN ACCORDANCE WITH AS 4678-2002.
 - CONTRACTOR TO ACCOUNT FOR ANY ADDITIONAL SURCHARGE LOADINGS PER RELEVANT STANDARDS.
 - GEOTECHNICAL REPORT DETAILS
CONSULTANT: WSP AUSTRALIA PTY. LTD. (WSP)
INVESTIGATION REF. NO.: PS212776-WSP-MWL-GEP-MEM-003 Rev2
INVESTIGATION DATE: 08/10/2024
SITE LOCATION: MACQUARIE POINT
 - EXISTING ROAD INFRASTRUCTURE TO BE PROTECTED FOR DURATION OF WORKS.

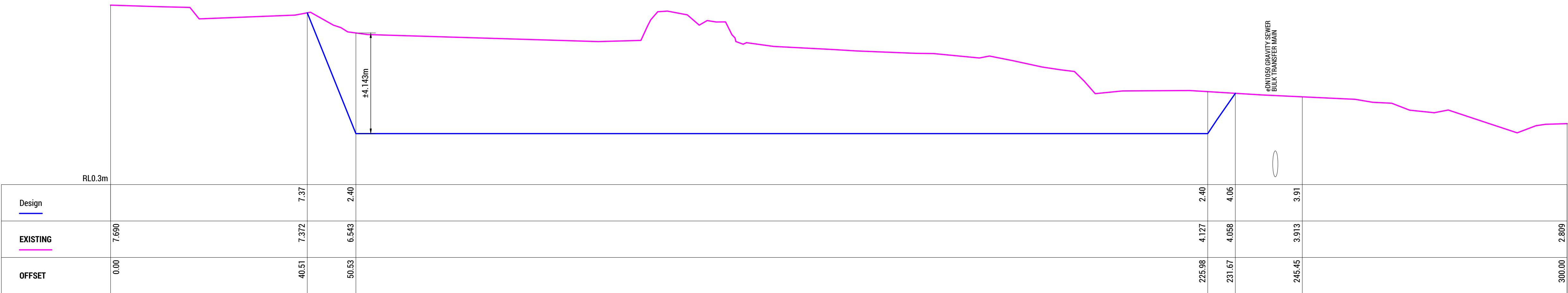
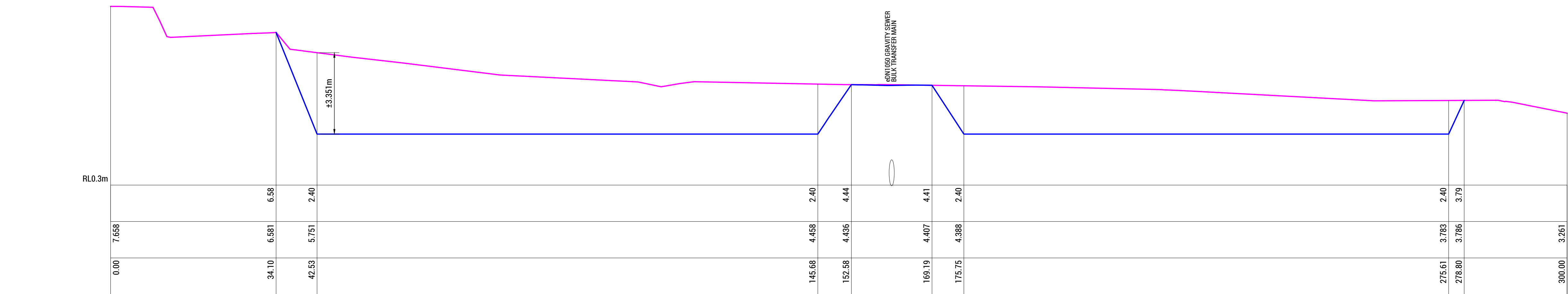
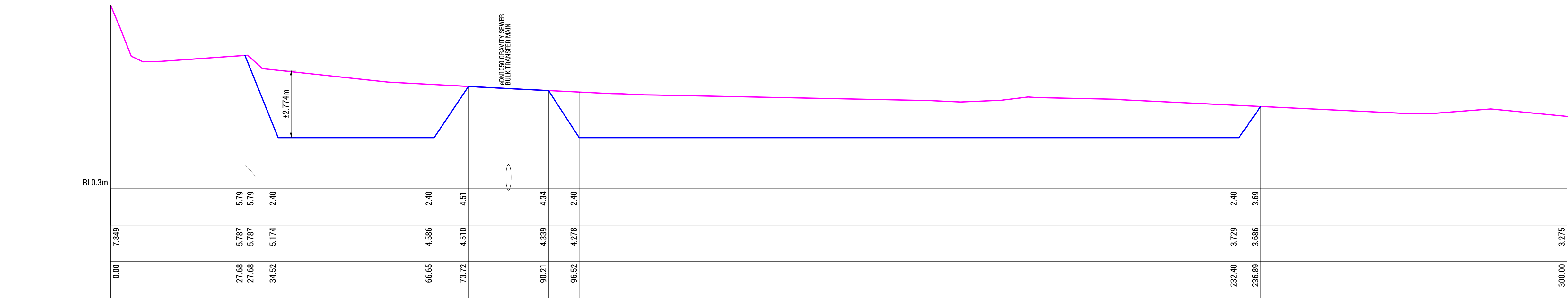
- LEGEND**
- eS EXISTING SEWER MAIN
 - eW EXISTING WATER MAIN
 - EXTENT OF EARTHWORKS
 - BATTERS - 1 IN 2 MAX.

NOTE:
THIS PLAN IS INDICATIVE ONLY. THE LOCATION
AND EXTENT OF EXISTING SERVICES SHOWN
ARE APPROXIMATE AND TO BE CONFIRMED.

					STATUS: PRELIMINARY / INFORMATION		DESIGN BY: -		 22-24 Paterson Street Launceston TAS 7250 rarein.com.au P. 03 6388 9200		CLIENT: MPDC		TITLE: BULK EARTHWORKS PLAN	
					DO NOT SCALE - IF IN DOUBT, ASK THIS DOCUMENT MAY ONLY BE USED FOR THE PURPOSE FOR WHICH IT WAS PREPARED. © RARE INNOVATION PTY LTD. ABN 51 619 598 257		DESIGN CHK: -				PROJECT: SITE EXCAVATION		SCALE: 1:1000 SHEET SIZE: A1 DWGS IN SET: -	
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							DRAFT CHK: -							
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Appendix B Design Parameters

Table 4 provides a summary of the value adopted (and the range of values that may be encountered). Fresh to slightly weathered Dolerite has been modelled as impenetrable material in SLOPE/W.

Table 4 *Geotechnical design parameters*

Unit	Unit name	Bulk unit weight, γ (kN/m ³)	Undrained shear strength, S_u (kPa)	Cohesion, c' (kPa)	Friction angle, Φ' (deg)
1A	Uncontrolled Fill (Granular)	17 to 19 (18)	-	0	30 to 35 (32)
1B	Uncontrolled Fill (Cohesive)	17 to 19 (18)	20 to 120 (40)	2 to 7 (3)	25 to 30 (28)
2	Estuarine (Granular)	16 to 18 (17)	-	0 to 2 (0)	28 to 34 (30)
3	Dolerite (MW-HW)	19 to 21 (20)	-	0	28 to 34 (34)

Note: Parameters reported as a range and recommended design value, i.e., Min to Max (Selected)



Appendix C Results Summary

Table 5 Summary of stability analysis results of expected geometries and ground conditions

Profile	Case ID		Factor of Safety	
			South - Evans St	North - Peace Promenade
		Section Ref	1	6
		Batter Height	1.6 m	4.2 m
		Offset from footpath	4.7 m	14 m
		Slope	2H:1V	2H:1V
Uncontrolled Granular Fill	a	Longer Term	1.49	1.28
	b	Short Term	1.41	1.28
	c	EQ	1.18	1.01 (1.95) ⁽¹⁾
Uncontrolled Cohesive Fill	d	Longer Term	2.28	1.66
	e	Short Term	4.83	2.74
	f	EQ	2.59	2.16
Uncontrolled Granular Fill overlying Estuarine soils	g	Longer Term	1.49	N/A
	h	Short Term	1.41	N/A
	i	EQ	1.18	N/A
Uncontrolled Cohesive Fill overlying Estuarine soils	j	Longer Term	2.28	N/A
	k	Short Term	4.44	N/A
	l	EQ	2.61	N/A

Note: (1) factor of safety noted in brackets refers to stability case in which the road or footpath is affected.

The analysis indicates that for all cases assessed where slope instability could affect the road reservation, the target FoS values are achieved. For Case ID c, (earthquake loading of the northern batter near the Peace Promenade) the stability is marginal, however the potential slip plane is shallow and would not affect the road reserve.

The sensitivity cases have been undertaken to assess the factor of safety (FoS) if the excavation is closer to the road reserve, or if there is some additional excavation depth.

Table 6 Summary of stability analysis results for sensitivity cases

Profile	Case ID		Factor of Safety					
			Southern - Evans St				North - Peace Promenade	
		Section Ref	2	3	4	5	7	8
		Batter Height	1.6 m	1.6 m	2.0 m	2.0 m	4.2 m	4.2 m
		Offset from footpath	2.0 m	0.0 m	4.7 m	2.0 m	4.0 m	14.0 m
		Slope	2H:1V	2H:1V	2H:1V	2H:1V	2H:1V	2.5H:1V
Uncontrolled Granular Fill	a	Longer Term	1.49	1.38	1.37	1.35	1.37	1.65
	b	Short Term	1.41	1.34	1.21	1.21	1.37	1.5
	c	Earthquake	1.18	1.12	1.08 (1.69) ⁽¹⁾	1.08 (1.39) ⁽¹⁾	1.08 (1.29) ⁽¹⁾	1.26
Uncontrolled Cohesive Fill	d	Longer Term	2.28	2	2.05	2.05	1.54	1.88
	e	Short Term	4.64	4.54	4.2	4.06	1.68	2.91
	f	Earthquake	2.7	2.77	2.39	2.48	1.45	2.22
Uncontrolled Granular Fill overlying Estuarine soils	g	Longer Term	1.49	1.38	1.37	1.37	N/A	N/A
	h	Short Term	1.41	1.34	1.22	1.22	N/A	N/A
	i	Earthquake	1.18	1.12	1.08 (1.67) ⁽¹⁾	1.08 (1.38) ⁽¹⁾	N/A	N/A
Uncontrolled Cohesive Fill overlying Estuarine soils	j	Longer Term	2.28	2.0	2.05	2.05	N/A	N/A
	k	Short Term	4.19	4.03	3.8	3.6	N/A	N/A
	l	Earthquake	2.6	2.61	2.37	2.35	N/A	N/A

Note: (1) factor of safety noted in brackets refers to stability case in which the road or footpath is affected.

The analysis indicates that for all cases assessed where slope instability could affect the road reservation, the target FoS values are achieved.

For earthquake cases, there are some minor non-conformances where the target FoS is not achieved, however these cases do not affect the road reserve.



Appendix D Limitations Statement



LIMITATION STATEMENT GEOTECHNICAL SITE INVESTIGATION AND GEOTECHNICAL INTERPRETATION

SCOPE OF SERVICES

This geotechnical interpretive report (the report) has been prepared in accordance with the scope of services set out in the contract, or as otherwise agreed, between the MPDC and WSP (scope of services). In some circumstances the scope of services may have been limited by a range of factors such as time, budget, site access and/or site disturbance constraints.

RELIANCE ON DATA

In preparing the report, WSP has relied upon data, surveys, analyses, designs, plans and other information provided by the client and other individuals and organisations, most of which are referred to in the report (the data). Except as otherwise stated in the report, WSP has not verified the accuracy or completeness of the data. To the extent that the statements, opinions, facts, information, conclusions and/or recommendations in the report (conclusions) are based in whole or part on the data, those conclusions are contingent upon the accuracy and completeness of the data. WSP will not be liable in relation to incorrect conclusions should any data, information or condition be incorrect or have been concealed, withheld, misrepresented or otherwise not fully disclosed to WSP.

GEOTECHNICAL INVESTIGATION

Geotechnical engineering is based extensively on judgment and opinion. It is far less exact than other engineering disciplines. Geotechnical engineering reports are prepared to meet the specific needs of individuals. A report prepared for a consulting civil engineer may not be adequate for a construction contractor or even some other consulting civil engineer. This report was prepared expressly for MPDC and expressly for purposes indicated by MPDC or their representative. Use by any other persons for any purpose, or by the client for a different purpose, might result in problems. MPDC should not use this report for other than its intended purpose without seeking additional geotechnical advice.

The WSP geotechnical and geophysics investigation services were conducted in a manner consistent with the level of care and skill ordinarily exercised by other members of the geotechnical and geophysical community currently practicing under similar conditions and subject to the time limits, financial and physical constraints applicable to the services.

THIS GEOTECHNICAL REPORT IS BASED ON PROJECT-SPECIFIC FACTORS

This geotechnical engineering report is based on a subsurface investigation which was designed for project-specification factors, including the nature of any development, its size and configuration, the location of any development on the site and its orientation, and the location of access roads and parking areas. Unless further geotechnical advice is obtained, this geotechnical engineering report cannot be used:

- when the nature of any proposed development is changed; and
- when the size, configuration location or orientation of any proposed development is modified.

This geotechnical engineering report cannot be applied to an adjacent site.

THE LIMITATIONS OF SITE INVESTIGATION

In making an assessment of a site from a limited number of investigation locations, there is the possibility that variations may occur between test locations. Site exploration identifies specific subsurface conditions only at those points from which samples

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have been taken. The risk that variations will not be detected can be reduced by increasing the frequency of test locations. The investigation program undertaken is a professional estimate of the scope of investigation required to provide a general profile of the subsurface conditions. The data derived from the site investigation program and subsequent laboratory testing are extrapolated across the site to form an inferred geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. Despite investigation, the actual conditions at the site might differ from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies.

The investigation logs are the subjective interpretation of subsurface conditions at a particular location, made by trained personnel. The interpretation may be limited by the method of investigation, and cannot always be definitive. For example, CPTu enables continuous profiling of soil properties along the test depth; however, its effectiveness is constrained by refusal in hard clay, dense sand/gravel or rock formations. In site investigation, the actual interface between materials may be more gradual or abrupt than a report indicates.

With reference to the geophysical assessment, Seismic refraction and multi-channel analysis of surface waves methods both have some inherent limitations in effectively representing subsurface conditions in all geological environments. The completed geophysical methods are remote sensing methods that may not detect all subsurface features. Depth of penetration is dependent on the nature of the subsurface. Furthermore, it is possible that interpreted features such as stratigraphic boundaries, top of bedrock, faults, voids, other geologic hazards and utilities may, upon intrusive sampling, prove to have been misinterpreted. Accurate interpretation of remote sensing data relies on the site-specific correlation of information with that obtained from borehole drilling, digging or other direct observation methods.

SUBSURFACE CONDITIONS ARE TIME DEPENDENT

Subsurface conditions may be modified by changing natural forces or man-made influences. A geotechnical engineering report is based on conditions which existed at the time of subsurface exploration.

Construction operations at or adjacent to the site, and natural events such as floods, or groundwater fluctuations, may also affect subsurface conditions, and thus the continuing adequacy of a geotechnical report. The geotechnical engineer should be kept appraised of any such events, and should be consulted to determine if additional tests are necessary.

AVOID MISINTERPRETATION

A suitably qualified geotechnical engineer should be retained to work with other appropriate design professionals explaining relevant geotechnical findings and in reviewing the adequacy of their plans and specifications relative to geotechnical issues.

BORE/PROFILE LOGS SHOULD NOT BE SEPARATED FROM THE ENGINEERING REPORT

Final bore/profile logs are developed by geotechnical engineers based upon their interpretation of field logs and laboratory evaluation of field samples. Customarily, only the final bore/profile logs are included in geotechnical engineering reports. These logs should not under any circumstances be redrawn for inclusion in architectural or other design drawings. To minimise the likelihood of bore/profile log misinterpretation, contractors should be given access to the complete geotechnical engineering report prepared or authorised for their use. Providing the best available information to contractors helps prevent costly construction problems. For further information on this matter, reference should be made to 'Guidelines for the Provision of Geotechnical Information in Construction Contracts' published by the Institution of Engineers Australia, National Headquarters, Canberra 1987.

GEOTECHNICAL INVOLVEMENT DURING CONSTRUCTION

During construction, excavation is frequently undertaken which exposes the actual subsurface conditions. For this reason geotechnical consultants should be retained through the construction stage, to identify variations if they are exposed and to conduct additional tests which may be required and to deal quickly with geotechnical problems if they arise.



REPORT FOR BENEFIT OF CLIENT

The report has been prepared for the benefit of MPDC and no other party. WSP assumes no responsibility and will not be liable to any other person or organisation for or in relation to any matter dealt with or conclusions expressed in the report, or for any loss or damage suffered by any other person or organisation arising from matters dealt with or conclusions expressed in the report (including without limitation matters arising from any negligent act or omission of WSP or for any loss or damage suffered by any other party not entitled to rely upon the factual information expressed in the report). Other parties should not rely upon the report or the accuracy or completeness of any conclusions (unless third-party reliance has been extended) and should make their own enquiries and obtain independent advice in relation to such matters.

OTHER LIMITATIONS

WSP will not be liable to update or revise the report to take into account any events or emergent circumstances or facts occurring or becoming apparent after the date of the report.